

Extreme dependence models and Environmental contours for safety assessment

Simulation et Optimisation pour les Energies Marines Renouvelables — Janvier 2018

Hélène Pineau², Marc Prevosto¹, Nicolas Raillard¹

January 11, 2018

1: IFREMER, Laboratoire Comportement des Structures en Mer — Marine Structures

2: Actimar, Brest





- 100-year return value of response is needed for design purpose ;
- **Metocean parameters impact** is fundamental ;
- **Multivariate setting** : waves, wind, currents ;
- 100-year return level for each parameter is too conservative ;
- Structure models are **too time-consuming** to obtain link between metocean variables and responses ;
- Focus here on **structure independent** methods.

Outline of the talk

1. Case study : tensions in mooring lines
2. Methodology
3. Results
4. Conclusion — Perspectives

Case study : tensions in mooring lines

Mooring lines tension

- Data provided by Engie, for Gjøa semi-submersible platform, 40km West offshore Norway, 100km North of Bergen ;

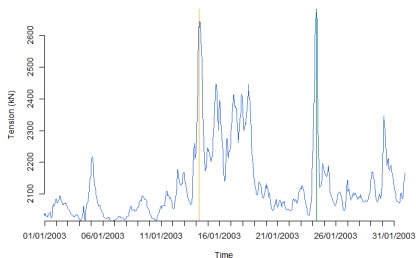
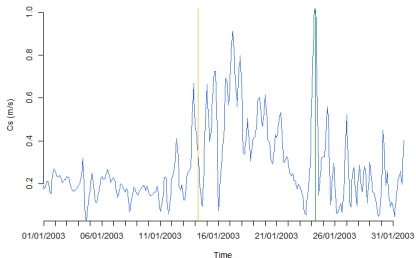
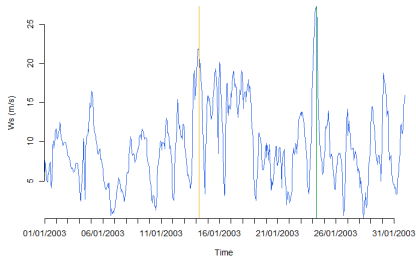
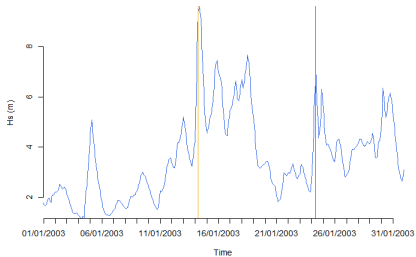


The Gjøa platform in the North Sea.
(Photo: Øyvind Nesvåg)

Source: <http://www.energy-pedia.com>

- Tensions in the mooring lines available during 20 storms, from 2011 to 2016 at 1Hz with many missing values ;
- Meta-model constructed to reproduce the observed data ;
- Retained parameters : H_s , W_s , C_s and their corresponding directions ;
- Reanalysis databases used to obtain synthetic response of the structure.

Mooring lines tension



Methodology

Extremal Values Modelling :
Methodology

Extremal Values Modelling : Methodology

- Hourly mooring line tension, from 1992/10 to 2015/12 ;
- POT modelling :
 - Extraction of storms : events above the 97.5% quantile, separated with at least 1 day ;
 - GPD hypothesis for the 99% quantile excess, parameters estimated with ML ;
 - Computation of the 100-years return level.
- Considered as the Golden Standard to reproduce ;
- Unavailable when designing new structures.

	Threshold	Nb. Obs.	Scale	Shape	100yr r.l.
Tension	2475.76	236	190.01	0.02	3761.36
Hs	7.03	194	1.75	-0.20	13.21
Ws	21.33	305	3.09	-0.18	33.41
Cs	0.88	292	0.20	-0.31	1.46

Table 1: Parameters of the fitted models and estimated return levels

Methodology

Extremal Values Modelling : dependency

- **Independence** : product of cdf ;
- Perfect dependence : 100-years RL occurs in all variables simultaneously ;

- Independence : product of cdf ;
- **Perfect dependence** : 100-years RL occurs in all variables simultaneously ;

- Standard method (e.g. DNV-GL, IEC-61400-3):
 - Weibull model for W_s ;
 - Weibull model for H_s given W_s ;
 - Log-normal distribution of T_p given H_s ;
- Linear model on the parameters of the distribution ;
- Only 2D ;
- No specific model for extreme, the bulk of the distribution is used.

- Classical approach in Structural engineering ;
- \mathbf{X} (physical space) \leftrightarrow \mathbf{Z} (normal space) ;
- Estimation : find Γ s.t.

$$\mathbb{P}(G_{\Gamma} > \mathbf{u}) = \mathbb{P}(Z_1 \geq u_1, Z_2 \geq u_2, Z_3 \geq u_3)$$

where $G_{\Gamma} \sim \mathcal{N}(0, \Gamma)$

Extreme value Dependence function

- Thresholds excess X (physical space) $\leftrightarrow Y$ (Frechet space) ;
- Probabilistic results on the joint p.d.f. of Y ;
- A.k.a **copula** model ;
- Parametric models can be used ;
- Estimation : **censored likelihood** ;
- Simulation is straightforward.

Conditional Model (Heffernan & Tawn)

- Thresholds excess X (physical space) $\leftrightarrow Z$ (Gumbel space)
- Non-linear regression model fitted using ML and assuming :

$$Z_{-i}|Z_i = \mathbf{a}_{-i|i}Z_i + Z_i^{\mathbf{b}_{-i|i}} \epsilon_{-i|i}, \text{ for } Z_i > \nu \text{ and } Z_i > Z_{-i}$$

where:

- Z_{-i} : all variables excluding Z_i ;
 - $\mathbf{a}_{-i|i}$ and $\mathbf{b}_{-i|i}$: parameters of the fitted pair-wise regression model ;
 - ν : dependency threshold ;
 - $\epsilon_{-i|i} \stackrel{i.i.d}{\sim} \mathcal{N}(\boldsymbol{\mu}_{-i|i}, \boldsymbol{\sigma}_{-i|i})$.
- Simulation :
 - Draw $\epsilon_{-i|i}$ (empirical, kernel smoothing, Gaussian...)
 - Rejection step to respect the empirical ratio of $Z_i > Z_{-i}$;
 - Transform back to original scale $Y_i = \exp(-F_i(Z_i))$.

Methodology

Environmental contours

Environmental contours

- Environmental contours : curves along which N-year levels of response will lie ;
- Contours are **independent of the structure** ;
- Hypothesis :
 - the ruin of the structure occurs in a single point in the space ;
 - The limit state is well approximated by an hyperplane.

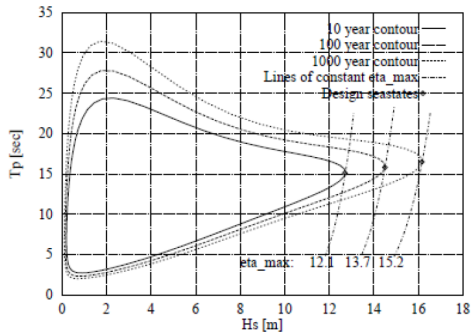


Figure 1: Example of Hs-Tp contours and extreme wave crest (η) as response (source: Wintershein-et-al 1993)

Estimating environmental contours

Huseby method

- Direct Monte Carlo simulations ;
- Works in the original space ;
- For any direction θ , one can estimate the hyperplane with p-probability of being exceeded ;
- The contour is the curve which tangents all these straight lines ;

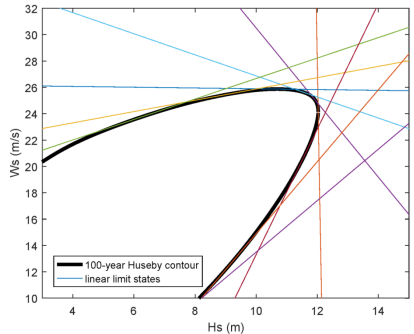


Figure 2: Huseby contour method

Results

Method	Tension	Rel. Err.	Hs	Ws	Cs
Meta model	3761.36	NA	NA	NA	NA
Independence	3536.22	-5.99	12.93	25.87	1.10
Perfect dependence	4152.32	10.39	13.21	33.41	1.46
Nataf	3911.71	4.00	13.02	32.11	1.28
Logistic model	3994.31	6.19	12.98	32.37	1.40
Conditional extremes	3627.88	-3.55	11.97	31.00	1.27

Table 2: Comparison of the methods for the estimation of 100-years return level.

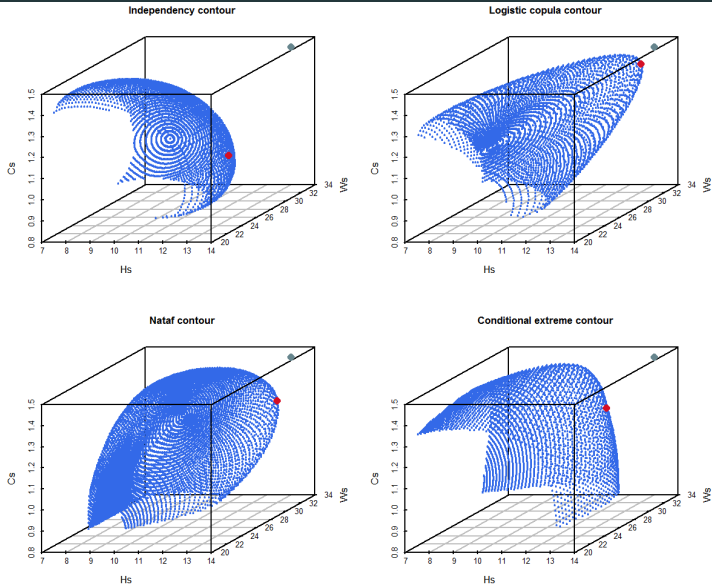


Figure 3: 3D contour and design points (red).

Conclusion – Perspectives

Conclusion — perspectives

- Comparison of multivariate methods to obtain extremal environmental conditions ;
- Both Parametric copula and Heffernan&Tawn performs well in 2D & 3D ;
- Procedure leading to realistic and less conservative design point ;

Future works perspectives :

- Higher dimensions and directional aspect ;
- Non-exchangeable model for copula ;
- Contours for coupled intensity variable and covariate (e.g. H_s and T_ρ) ;